

## Soil investigation

**Sanjeev Kattimani<sup>1</sup>, Kavya MPM<sup>2</sup>, S Parashivamurthi<sup>3</sup>, Sharanabassu<sup>4</sup>, Shantinath<sup>5</sup>**

<sup>1,3,4,5</sup> Students, Department of Civil Engineering, Rajarajeswari College of Engineering, Karnataka.

<sup>2</sup>Asst. Professor, Department of Civil Engineering, Rajarajeswari College of Engineering, Karnataka.

\*\*\*

**Abstract** - civil engineering practice faces many challenges and encounters with soil which is used as foundation to support the structures and also commonly used as construction material. This fact necessitates a detailed study of the geological and geotechnical characteristics of the soil, so that a successful solution can be obtained for the various problems for the structures; arising due to the poor and undesirable Geological The and engineering properties of the supporting soil. Geologists work hand in hand with civil engineers particularly geotechnical engineers in almost all areas of civil engineering field. Geology aids the geotechnical engineer owing to the fact that the method of formation of soil mass influences its size, shape and behavior.

### 4. GENERALIZE BORE HOLE LOG(SUBSURFACE)

Table 1:

Borehole identity	Generaliz e depth m	Soil characteristic
BH#1	0.00-4.5	Light brownish to greyish medium dense fine silty sand
	4.5-15	Greyish to yellowish medium plastic very stiff to hard silty clay.
BH#2	0-1.5	Greyish to yellowish loose medium dense fine silty sand
	1.5-15	Greyish medium dense to fine silty sand.

Table 2:

	Genera lize depth m	Generaliz e soil characteri stics	N-design	q- desi gn	c- design,K N/m
1	0.0- 4.5	Loose to medium dense fine silty sand	11	26	0
2	4.5- 15.0	Stiff to very stiff medium plastic silty clay	21	20	20
3	15 onwar ds	Medium dense fine to coarse sand	24,25,26,...(Nt h+1)...	32	0

### 1. INTRODUCTION

The objective of the report is restricted to the factual information to be collected investigation period along with laboratory tests results and so as to obtain sequence & extent of soil so as to arrive at design parameters for the foundations from the recommended safe bearing capacity of foundation soil.

### 2. LITERATURE REVIEW

Soil investigation and laboratory testing are presented and the vibro design principle in particular use of a modification of soil around the stone column are outlined, the effect of subsurface investigation on construction cost over runs by examining the result from the database and subsurface condition cost overruns increase with decreases site investigation expense related to total construction cost.

### 3. METHODOLOGY

Keeping in mind the general objectives of the project a detailed methodology for undertaking the project is proposed which consists of a set task as given below.

The laboratory tests on soil samples were started immediately after the receipt of the same in the laboratory. Following laboratory tests are carried out to determine the physical and engineering properties of undisturbed and disturbed soil samples.

1. Dry density and moisture content (IS 2720 part - 2 & 29)
2. Particle size analysis - (IS 2720 part - 4 1985)
3. Atterberg's limit - (IS 2720 part - 5 1985)
4. Specific gravity - (IS 2720 part - 3/sec2 1980)

5. RESULTS AND DISCUSSION



5.1. LABROTARY TEST FOR BOREHOLE -1

TEST CONDUCTED	RESULTS
Alterberg's Limit	Liquid limit(graph)=48% Flow index(graph)=227 Plastic limit=27.2% Plasticity index=21.3% Liquidity index=0.263 Consistency index=0.734 Toughness index=0.94
Specific gravity	2.66
Water content	7.8%



TEST CONDUCTED	RESULTS
Alterberg's Limit	Liquid limit=49% Flow index=229 Plastic limit=28.4% Plasticity index=22.1% Liquid index=0.294 Consistency index=0.845

	Toughness index=0.97
Specific gravity	2.71
Water content	8.1%

5.2. LABROTARY TEST FOR BOREHOLE-2

Table 4.3.Field Test for Borehole-1

Boring depth in m	Description of strata	Sample type number (m)	Field tests type results	Remarks
0.0	Reddish brown clay silt with sand	Ds @ 0.5		
1.5	Brownish white silty sand	Spt @ 1.5	6,7,8	N=15
3.0	Reddish yellow to white silty sand	Spt @ 3	9,10,12	N=22
4.5	Reddish yellow to white silty sand	Spt @4.5	10,11,15	N=26
6.0	Brownish white silty sand	Spt @6	12,14,16	N=30
7.5	Greenish brown to white sandy slit	Spt @7.5	14,17,19	N=36
9.0	Brownish white silty sand	Spt @9	15,18,20	N=38
10.5	Brownish white silty sand	spt@10.5	18,22,29	N=51
12.0	Brownish white CWR	Spt @12	30,34,46	N=80
13.5	Brownish white CWR	Spt @13.5	46,50b/70	N>100 RS
15.0	Brownish white CWR	Spt @15	50b/R	N>100 RS

Table 4.4: Field Test Borehole-2

Boring depth in m	Description of strata	Sample type number (m)	Field tests type results	Remarks
0.0	Reddish brown clay silt with sand	Ds @ 0.5		
1.5	Brownish white silty sand	Spt @ 1.5	6,7,9	N=16
3.0	Reddish yellow to white silty sand	Spt @ 3	9,11,12	N=23
4.5	Reddish yellow to white silty sand	Spt @4.5	10,13,15	N=28
6.0	Brownish white silty sand	Spt @6	12,15,17	N=32
7.5	Greenish brown to white sandy slit	Spt @7.5	14,16,19	N=35
9.0	Brownish white silty sand	Spt @9	16,18,22	N=40
10.5	Brownish white silty sand	spt@10.5	18,25,30	N=55

12.0	Brownish white CWR	Spt @12	30,35,46	N=81
13.5	Brownish white CWR	Spt @13.5	46,50b/70	N>100 RS
15.0	Brownish white CWR	Spt @15	50b/R	N>100 RS

## 6. CONCLUSIONS

Sub soil stratification of proposed site up to depth of investigation is as below from 0.0 to 0.3m: Brownish Black Silty Clay of High Plasticity with Little Gravel

From 0.3 to 5.1m: Brownish Yellow Very Stiff Silty Clay of Medium Plasticity with Little Gravel from 5.1 to 10.0m: Yellow Hard Silt Clay of Low Plasticity with Little Gravel

Looking to the proposed type of project RCC Circular Raft footing is recommended along with their Safe Bearing Capacities considering factor of safety of 2.5 as shown in Table - I.

Safe Bearing pressure calculated for 100mm permissible as per I.S 8009 Part I for over consolidated clay as shown in Plate-4

Water table was considered in the analysis of SBC.

Suitability of Soil for back filling: The top layer of soil is of High swelling characteristic, which is not suitable for back filling purpose.

## 7. ACKNOWLEDGEMENT

It also gives us immense pleasure to express our gratitude to **Ms Kavya MPM** Assistant Professor of Civil Engineering whose valuable inputs have made us richer in terms of knowledge and also for guiding us at a place where everything was not familiar, her supervision during our project has been proved as the greatest asset to our project.

## 8. FUTURE SCOPE

Reconnaissance / field trip for studying the general topography and geology of the area/ terrain The field Geotech investigations consisted of conducting 18.0 nos. of bore holes for SPT/DCPT up to maximum depth of 6.0 m or refusal and 14.0 nos. of DCPT upto maximum explored depth of 6.0 m or refusal, below N.S.L whichever is

earlier as per IS code Conducting SPT/DCPT in the bore-hole/trial pits at regular intervals and collecting disturbed/undisturbed soil samples from the bores hole at regular intervals and conducting field density tests as per Indian code of practice.

Conducting Plate Load Test using 75 cm square plate at 1 no of locations and Collection of Disturbed & Undisturbed soil

## 9. REFERENCES

- Gould, J P., "Geotechnology in Dispute Resolution," Journal of Geotechnical Engineering , vol.121,No, 7,July 1995,pp. 523-534
- Baynes, F.J, "Sources of Geotechnical Risk," Quarterly Journal of engineering Geology, Vol, 43, 2010,pp321-331.
- Badger, T., "State DOT Field Exploration Programs Survey of current Practice and Utilization," presented at the 94<sup>th</sup> Annual Meeting of the Transportation Research Board, Washington., D C., Jan 2015..
- Khan, A., "Geotechnical Changes Orders During Construction," Presented at the 43<sup>rd</sup> Annual FHWA Midwest Geotechnical Conference, Bloomington, Minn., oct 2014.
- Mayne, P. W., NCHRP Synthesis of Highway Practice 368 Cone Penetration Testing, Transportation Research Board of the National Academics, Washington. D,C 2001,332pp.