

Seismic Analysis of Asymmetric RCC Buildings using Passive Energy Dissipating Devices

Shruti S. Mane¹, Dr. Y. M. Ghugal²

¹PG Student, Government College of Engineering, Karad, Maharashtra, India

²Head of Department, Applied Mechanics, Government College of Engineering, Karad, Maharashtra, India

Abstract - This paper deals with the study of different passive energy dissipation devices. In the present study, the effectiveness of dampers in reducing the dynamic responses of a structure under seismic excitations is evaluated analytically using response spectrum method. Three different dampers viz., Visco Elastic dampers, Fluid Viscous dampers and Friction dampers are used to evaluate the response of asymmetric RCC buildings. G+10 storied four different buildings with asymmetric plans i.e., C shaped, T shaped, L shaped and U shaped are considered for the analysis. A parametric study for finding optimum damper properties for the reinforced concrete frames was conducted. In response spectrum method, earthquake load is applied in both x and y direction. For the analysis purpose Etabs v.16 software is been used by considering seismic zone IV as per IS 1893:2002(part 1) code. Results of these analyses are discussed in terms of various parameters such as storey displacement, storey drift, base shear, bending moment, axial forces. The analysis results confirmed that a significant reduction in the responses such as displacements and other forces is possible with the introduction of dampers and hence it can be used as an alternative to the conventional ductility based design method of earthquake resistant structures.

Key Words: Passive dampers, fluid viscous dampers, viscoelastic dampers, friction dampers, response spectrum

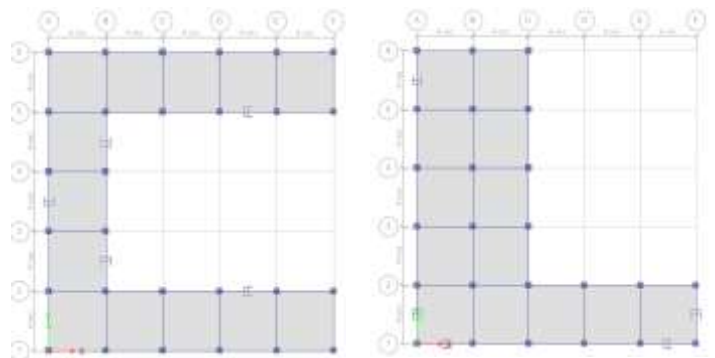
1. INTRODUCTION

Passive energy dissipation is a rising technology that enhances the performance of the building by adding damping (and in some cases stiffness) to the building. The principal use of energy dissipation devices is to reduce earthquake displacement of the structure. Energy dissipation devices will also decrease force in the structure provided the structure is responding elastically but would not be expected to reduce force in structures whose response is beyond yield. For many applications, energy dissipation provides an alternate approach to standard stiffening and strengthening schemes and would be expected to achieve comparable performance levels. Other objectives may additionally influence the choice to use energy dissipation devices, since these devices may also be helpful for control of building response due to minute earthquakes, wind, or mechanical vibrations.

2. BUILDING DESCRIPTION

This study consists G+10 storied building of C shaped, L shaped, U shaped and T shaped plans along with dampers are as shown in the Fig 1. Models consist of RCC building having 5 bays in each principal direction; each bay is having width of 4 m.

1. Grade of concrete: M25
2. Grade of steel for longitudinal bars: HYSD415
3. Grade of steel for confinement bars (Ties): Fe250
4. Floor to floor height: 3 m
5. Slab thickness: 120mm
6. Size of Column: 450mm×450mm
7. Size of Beam: 450mm×300mm
8. Seismic Zone: V
9. Soil condition: Medium
10. Importance Factor: I
11. Damping: 5%
12. Building Frame: Special moment resisting frame (SMRF)
13. Density of concrete: 25 kN/m³
14. Types of damper: Visco-elastic, Fluid viscous and Friction dampers.



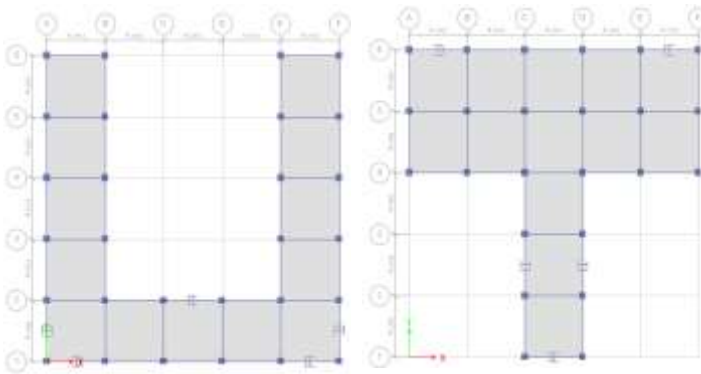


Fig -1: Plans of the buildings considered for the study

3. DAMPER PROPERTIES

3.1 Visco Elastic damper

Visco elastic damper of 3M Company is used for the analysis. The model considered is ISD110. Properties derived are as follows;

Table -1: Properties of VED

Stiffness of damper	12161.6 kN/m
Damping Coefficient	14593.92 kNs/m

3.2 Fluid Viscous Damper

Fluid Viscous Damper of Taylor Devices Inc. is used for the analysis. The model considered is 17190. Properties derived are as follows;

Table -2: Properties of FVD

Stiffness of damper	149883 kN/m
Damping Coefficient	10048 kNs/m
Damping exponent	0.45

3.3 Friction Damper

Friction damper of Pall Dynamics Limited is used for the analysis. Properties derived are as follows;

Table -3: Properties of FD

Stiffness of damper	54460.408 kN/m
Yield strength	700 kN
Post Yield Stiffness Ratio	0.0001
Yielding Exponent	10
Slip Load	500 kN
Brace section	ISMB 200

4. MODELING AND ANALYSIS

Modelling and analysis includes modelling of different building elements, different load cases considered for analysis and method used for analysis.

4.1 Modeling in ETABS

The building is modeled using finite element software ETABS. The following models are considered for C shaped, L shaped, U shaped and T shaped buildings for the analysis;

WD: Model without damper

VED: Model with visco elastic damper

FVD: Model with fluid viscous damper

FD: Model with friction damper

4.2 Method of analysis

The seismic analysis of all buildings is carried by Response Spectrum Method in accordance with IS: 1893 (Part I): 2002. Dynamic response of these buildings in terms of base shear, modal time period, storey displacement, storey drift, axial force in column, bending moment and torsional moment in beams.

5. RESULTS AND DISCUSSION

3D analysis has been carried out by using response spectrum method. Models are analysed to study the performance of structure by considering effect of three dampers in four different asymmetric RCC buildings. Dynamic response of these buildings is presented in terms of base shear, time period, storey displacement, storey drift, torsional moments, and axial forces in column.

5.1 Results for C shaped building

By observing the performance of C shaped building installed with visco-elastic damper, fluid viscous damper and friction damper, visco-elastic damper is found to be more effective in reducing displacement, drift, base shear, axial force and torsional moments as compared to other dampers installed in the building at the considered location. The results are shown using graphs shown in below charts;

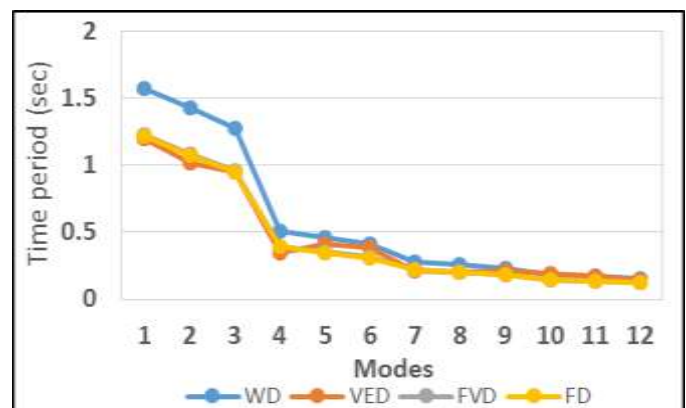


Chart -1: Variation of time period

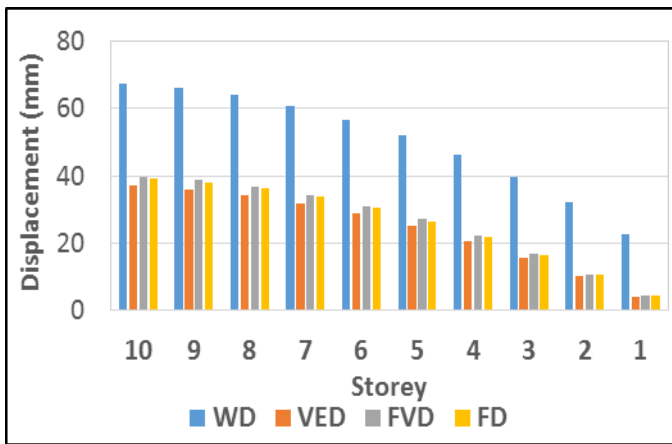


Chart-2: Variation of storey displacement

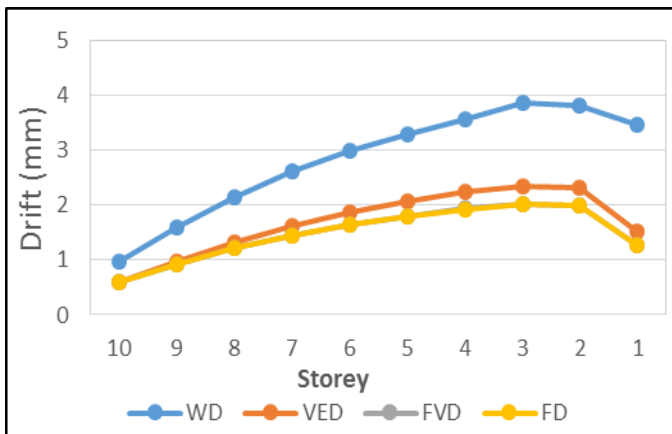


Chart-3: Variation of storey drift

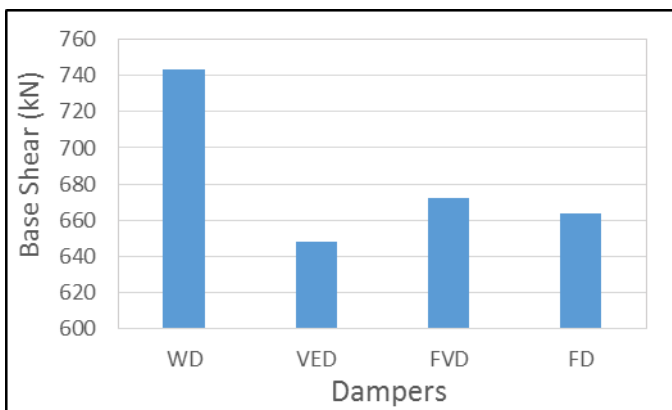


Chart-4: Variation of base shear

The results for axial force in column and torsional moment in beam are presented for the considered column and beam as shown in below fig 2.

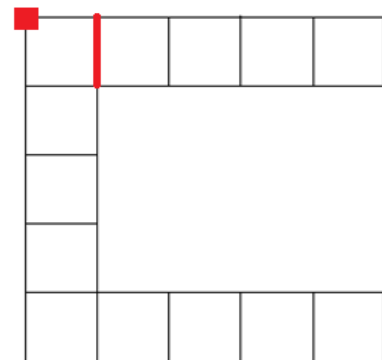


Fig-2: Column and beam considered for results

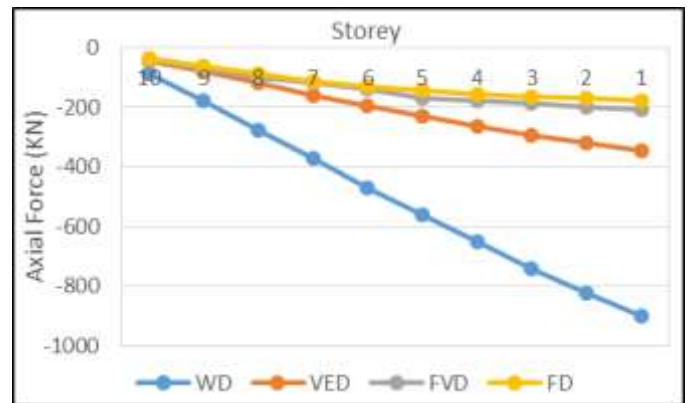


Chart-5: Variation of axial force in column

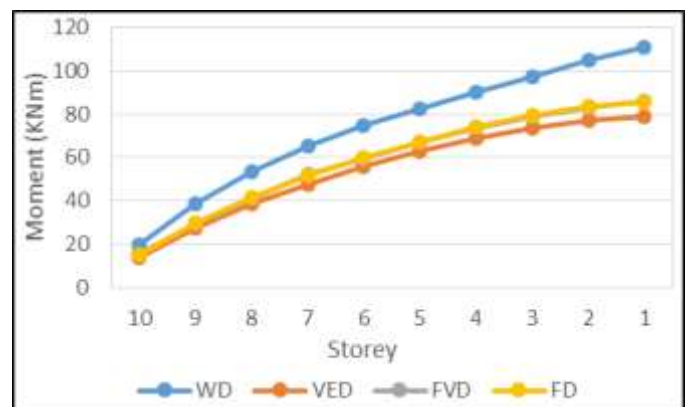


Chart-6: Variation of torsional moment in beam

5.2 Results for L shaped building

From the analysis of L shaped building installed with visco-elastic damper, fluid viscous damper and friction damper, it is found that visco elastic damper gives less storey displacement, storey drift, base shear, axial force and torsional moments as compared to other dampers. The results for axial force in column and torsional moment in beam are presented for the same column and beam in L shaped building as shown in fig 2 for C shaped building. The results are shown using graphs shown in below charts;

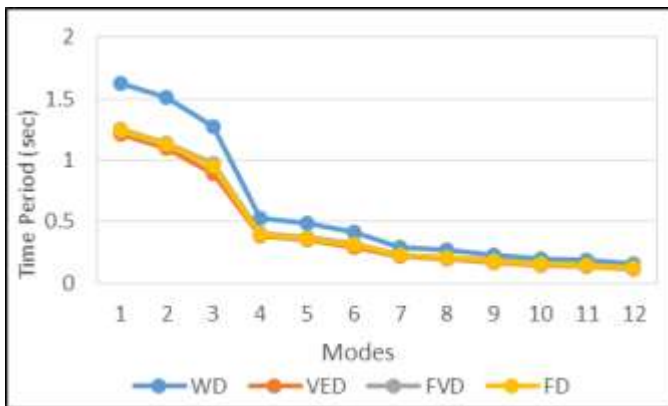


Chart -7: Variation of time period

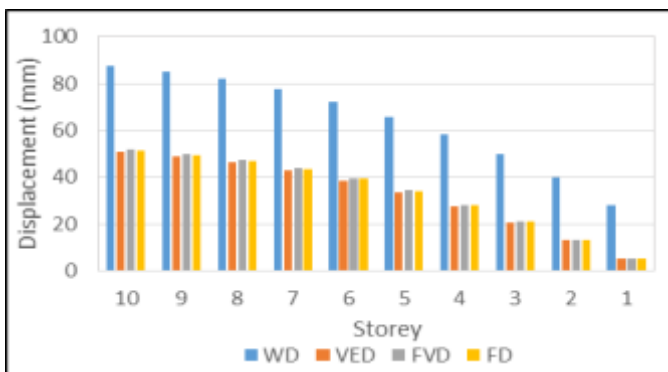


Chart -8: Variation of storey displacement

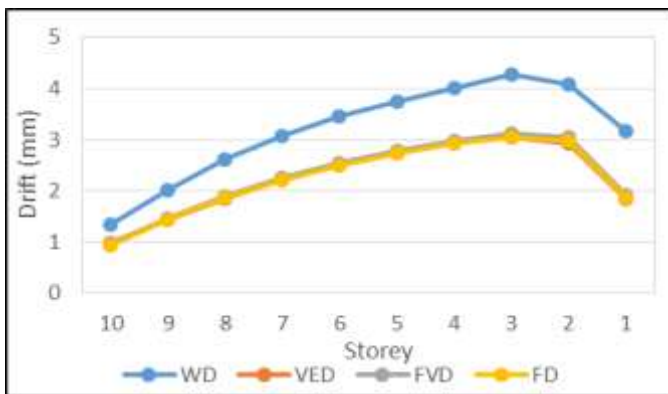


Chart -9: Variation of storey drift

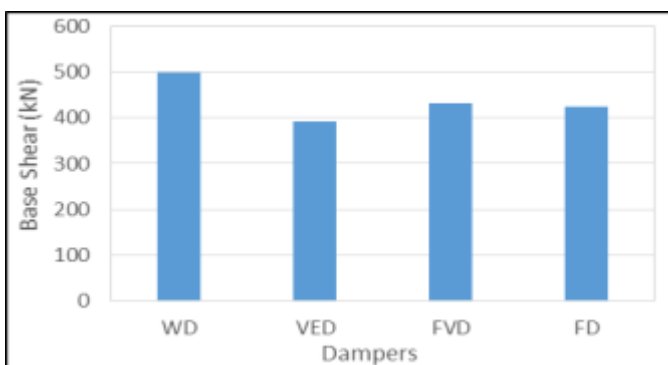


Chart -10: Variation of base shear

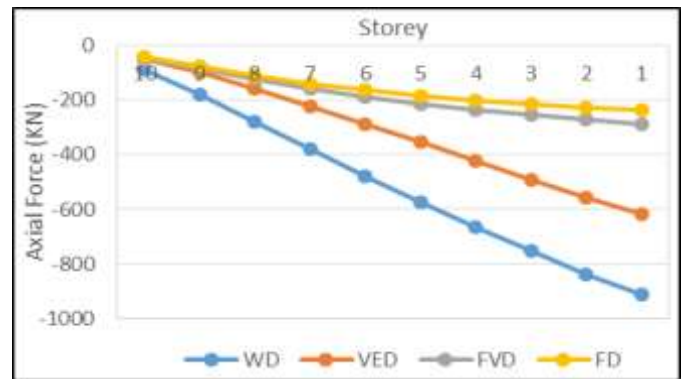


Chart -11: Variation of axial force in column

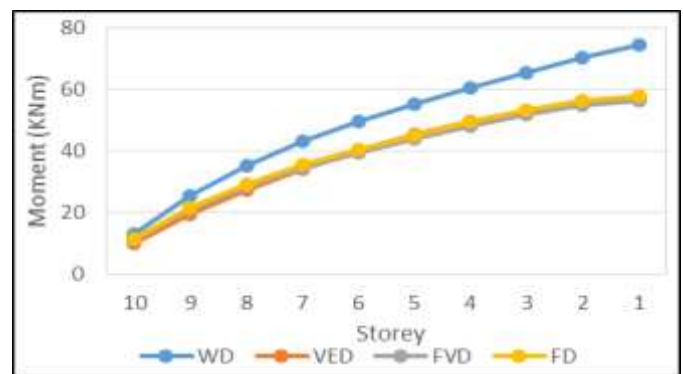


Chart -12: Variation of torsional moment in beam

5.3 Results for U shaped building

For U shaped building after analysis, it is seen that the behaviour of friction damper is more effective as compared to visco-elastic damper and fluid viscous damper. The results obtained by installation of friction damper for the considered location of dampers has shown large reduction in displacement, drift, base shear, axial force, column moments, beam bending moments and torsional moments as compared to building without damper. The results for axial force in column and torsional moment in beam are presented for the same column and beam in U shaped building as shown in fig 2 for C shaped building. The results are shown using graphs shown in below charts;

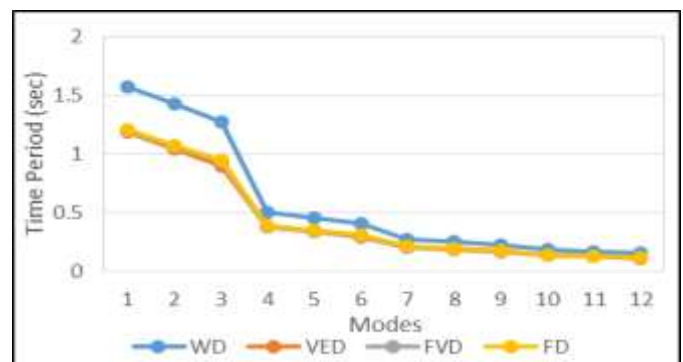


Chart -13: Variation of time period

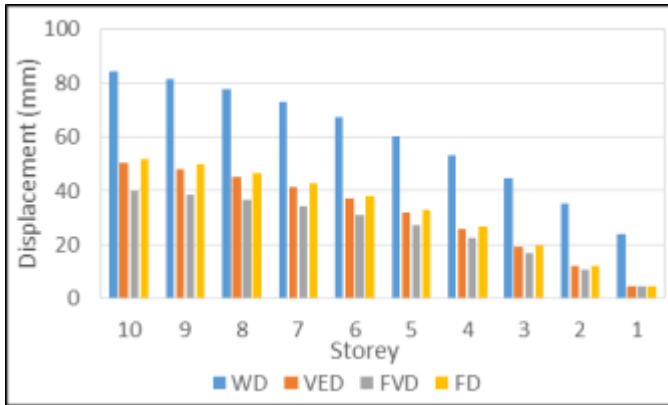


Chart -14: Variation of storey displacement

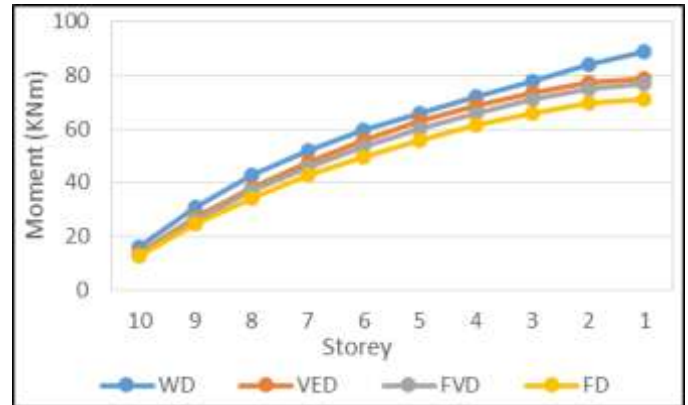


Chart -18: Variation of torsional moment in beam

5.4 Results for T shaped building

After carrying out the analysis of T shaped buildings, it is seen that visco-elastic damper show large reductions in the results as compared to fluid viscous damper and friction damper. Visco-elastic damper prove better than the other two dampers. The results for axial force in column and torsional moment in beam are presented for the same column and beam in T shaped building as shown in fig 2 for C shaped building. The results are shown using graphs shown in below charts;

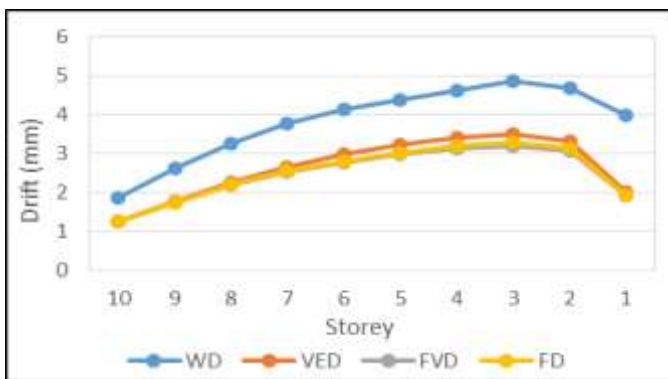


Chart -15: Variation of storey drift

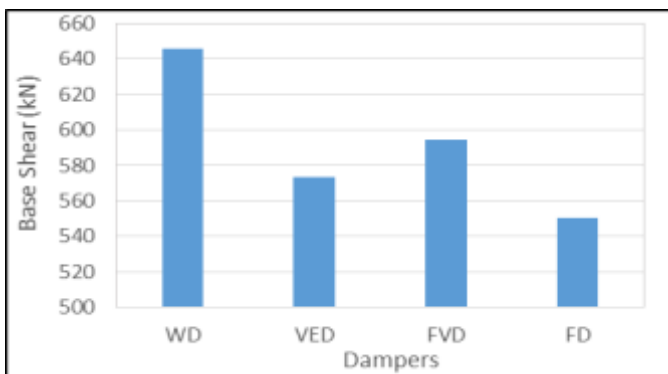


Chart -16: Variation of base shear

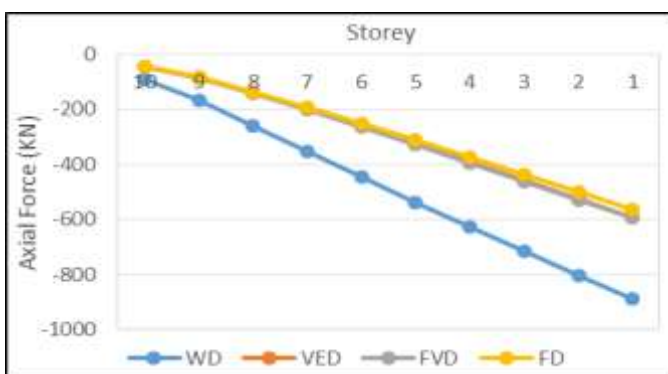


Chart -17: Variation of axial force in column

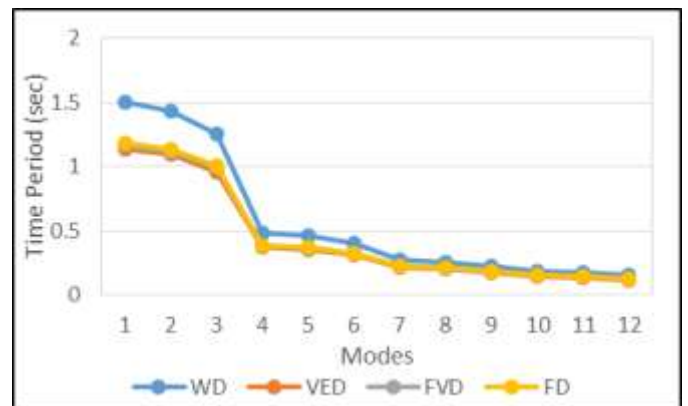


Chart -19: Variation of time period

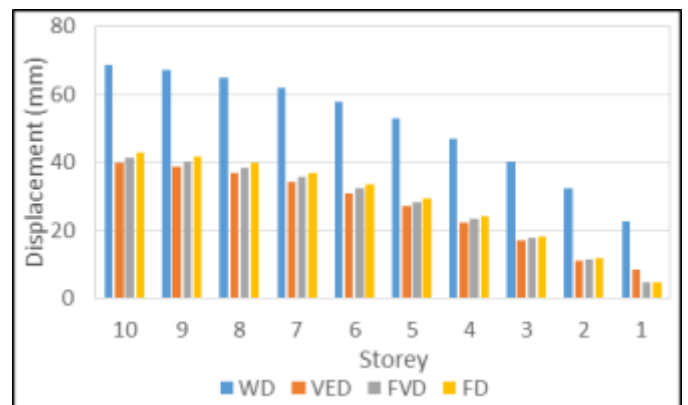


Chart -20: Variation of storey displacement

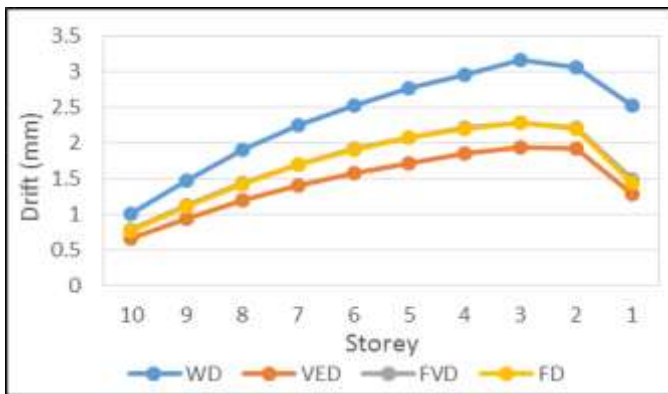


Chart -21: Variation of storey drift

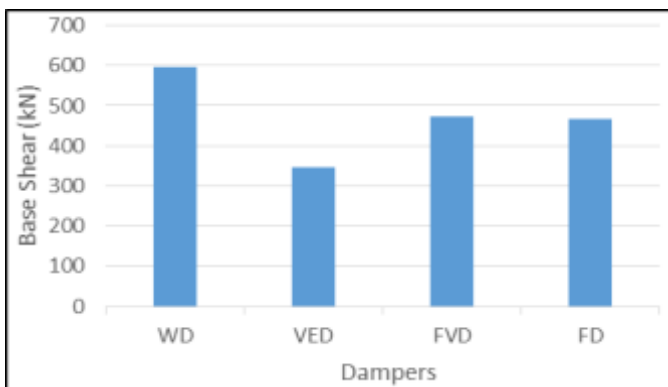


Chart -22: Variation of base shear

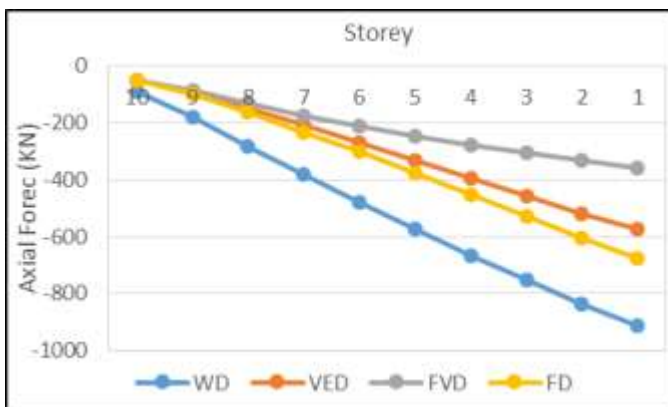


Chart -23: Variation of axial force in column

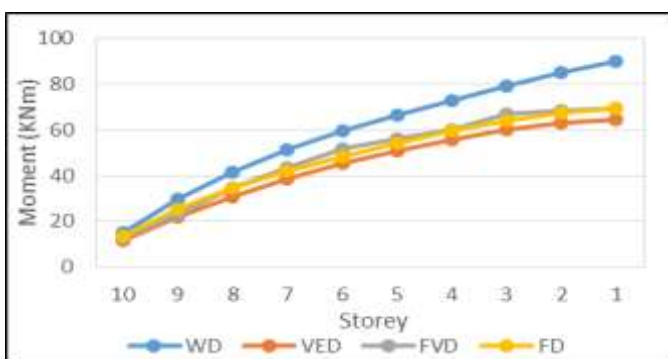


Chart -24: Variation of torsional moment in beam

6. CONCLUSIONS

After analysis the seismic performance of different models has been compared and effectiveness of passive energy dissipating devices is concluded. The conclusions of the present study are as follows:

1. After addition of the dampers to the building, a significant performance improvement is achieved. The responses of the storey displacement and inter-storey drifts decreased significantly as a result of added dampers.
2. It can be concluded that with the application of dampers in the structure maximum response, base shear reduces in structure. The time period is also found to be reduced as compared to that of bare frame with the addition of dampers. The bending moments in beams, axial forces in columns are greatly reduced with the addition of dampers.
3. It is found that the performance of the building structure in seismic loading is improved to great extent with the installation of dampers. From analysis it can be seen that it is necessary to properly implement and construct a damper in any high rise building situated in earthquake prone areas.
4. By the proper selection of location of dampers, the response of the structure can be minimized to great extent.

REFERENCES

1. Constantinou. M. C, Symans. M. D, Tsopelas. P and Taylor. D. P, "Fluid Viscous Dampers in Applications of Seismic Energy Dissipation and Seismic Isolation", ATC-17-1 Seminar on Seismic Isolation, Passive Energy Dissipation and Active Control, San Francisco, CA, 1993, pp. 581-592.
2. Mcnamara. R. J and Taylor. D. P, "Fluid Viscous Dampers for high-rise buildings", Structural Design of Tall and Special Building, Vol 12, (2003), pp. 145-154
3. Kokil. A. S and Shrikhande. M, "Optimal Placement of Supplemental Dampers in Seismic Design of Structures", Journal of Seismology and Earthquake Engineering, Vol. 9, No. 3, 2007, pp. 125-135.
4. Lin. W. H and Chopra. A. K, "Earthquake Response of Elastic SDF Systems With Non-Linear Fluid Viscous Dampers", Earthquake Engineering and Structural Dynamics, Vol-31, 2002, pp. 1623-1642.
5. Sorace. S and Terenzi. G, "Seismic Protection of Frame Structures by Fluid Viscous Damped Braces", American Society of Civil Engineers, Journal of Structural Engineering, Vol. 134, No. 1, January 1, 2008, pp 45-55.
6. Ras. A and Boumechra. N, "Seismic energy dissipation study of linear fluid viscous dampers in steel structure

- design", Alexandria Engineering Journal, Production and hosting by Elsevier B.V., 2016.
7. Guo. T, Xu. J, Xu. W and Di. Z , "Seismic Upgrade of Existing Buildings with Fluid Viscous Dampers: Design Methodologies and Case Study", American Society of Civil Engineers, Journal of Structural Engineering, October 1, 2014, pp. 04014175 (1-11)
 8. Diclelia. M and Mehta. A, "Seismic performance of chevron braced steel frames with and without viscous fluid dampers as a function of ground motion and damper characteristics", Journal of Constructional Steel Research, Vol.63, 2007, pp.1102-1115.
 9. Miyamoto. K., and Singh.J. P. (2002). "Performance of structures with passive energy dissipators." Earthquake Spectra, Earthquake Engineering Research Institute, Volume 18, February 2002, pp. 105-119.
 10. Wang. S and Mahin. S. A., "Seismic Upgrade of an Existing Tall Building Using Different Supplemental Energy Dissipation Devices", American Society of Civil Engineers, Journal of Structural Engineering, Vol. 144(7), No. 1, May 14, 2018, pp. 04018091 (1-11).
 11. Constantinou, M.C. And Symans, M. D. "Experimental And Analytical Investigation Of Seismic Response Of Structures With Supplemental Fluid Dampers", Report No. NCEER 92-0032, National Centre for Earthquake Engineering Research, University Of New York At Buffalo, Buffalo, NY, 1992.
 12. Baratia. F and Esfandiari. A, "Exploring the Efficiency of Dampers for Repair and Strengthening of Existing Buildings", Journal of Structural Engineering and Geotechnics, 2 (2), Summer 2012, pp 67-73
 13. Chandra. R, Masand. M, Nandi. S. K, Tripathi. C. P, Pall. R and Pall. A, "Friction-Dampers For Seismic Control Of La Gardenia Towers South City, Gurgaon, India" Proceedings, 12 World Conference on Earthquake Engg., 2008.
 14. Pall.S, and Marsh.C. "Response of friction damped braced frames" Proceedings American Society of Civil Eng Vol. 108(60), No. ST6, June, 1982, pp.1313-1323.
 15. Shao, D., Pall, A. and Soli, B. 2006. "Friction Dampers for seismic upgrade of a 14-story patient tower with a 36-foot tall soft story." Proc. of 8th U. S. National Conf. on Earthquake Engineering, EERI, Oakland, Calif., Paper No. 90
 16. Borislav. B. and Mualla. I. H, "Performance of Steel Frames with a new Friction Damper Device under Earthquake Excitation", Engineering Structures, Production and hosting by Elsevier B.V, Vol. 24, 2002, pp. 365-371.
 17. Tabeshpour. M. R and Ebrahimian. H, "Technical Note on Seismic Retrofit of Existing Structures Using Friction Dampers", Asian Journal of Civil Engineering (Building and housing) Vol. 11, No. 4, 2010, pp. 509-520.
 18. Aiken. I. D, Nims. D. K, Whittaker. A. S, and Kelly. J. M, "Testing of Passive Energy Dissipation Systems", Earthquake Spectra, Vol. 9, No. 3, Earthquake Engineering Research Institute California, August (1993)
 19. Tezcan.S. S, Uluca.O, "Reduction of earthquake response of plane frame buildings by viscoelastic dampers", Engineering Structures, Production and hosting by Elsevier B.V., Vol No- 25, 21 July 2003 , pp. 1755-1761.
 20. Min. K. W, Kimb. J, and Lee. S. H, "Vibration tests of 5-storey steel frame with viscoelastic dampers", Engineering Structures 26 (2004), Production and hosting by Elsevier B.V, pp. 831-839.
 21. Li. C and Reinhorn. A. M (1995), "Experimental and analytical investigation of seismic retrofit of structures with supplemental damping: Part II-Friction devices", Technical report NCEER-95-0009. Buffalo (NY): State University of New York at Buffalo.
 22. Marko. J, Thambiratnam. D and Perera. N, "Study of Viscoelastic and Friction Damper Configurations in the Seismic Mitigation of Medium-Rise Structures", Journal of Mechanics of Materials and Structures, Vol 1, No 6, June 2006, pp. 1001-1039.
 23. Pong W.S, Tsai. C. S and Lee. G. C., "Seismic Performance of High-Rise Building Frames with Added Energy-Absorbing Devices" National centre for earthquake engineering research, ISSN 1088-3800, June 20, 1994.
 24. Vijay. U. P, Rajkumar. P. R. and Ravichandran. P. T, "Seismic Response Control of RC Structure using ViscoElastic Dampers", Indian Journal of Science and Technology, Vol 8(28), October 2015.
 25. Zhang, R. H., Soong, T. T. And Mahmoodi,P., "Seismic Response Of Steel Frame Structures With Added Viscoelastic Dampers", Earthquake Engineering and Structural. Dynamic, Vol-18, 1989, pp. 389- 396.
 26. Aiken, I. D. And Kelly, J. M., "Earthquake Simulator Testing and Analytical Studies of Two Energy-Absorbing Systems for Multi-Storey Structures", Report No. UCB/EERC-90/03, Earthquake Engineering Research Centre, University Of California at Berkeley, Berkeley, CA, 1990.
 27. Tovar.C and Lopez.O.P, "Effect of the Position and Number of Dampers on the Seismic Response of Frame Structures", 13th World Conference on Earthquake Engineering Vancouver, B.C., Canada, August 1-6, 2004, Paper No. 1044

28. Symans. M. D, Charney. F. A, Whittaker. A. S, Constantinou. M. C, Kircher. C. A, Johnson. M. W, and McNamara. R. J, "Energy Dissipation Systems for Seismic Applications: Current Practice and Recent Developments", American Society of Civil Engineers, Journal of Structural Engineering, Vol. 134, No. 1, January 1, 2008.
29. Soong T.T and Spencer. B. F, "Supplemental Energy Dissipation: State-of-the-Art and State of-the Practice", Engineering Structures, Production and hosting by Elsevier B.V, Vol.24, 2002, pp. 243–259.
30. Filiatrault, A., Tremblay, R., and Wankitkorkul, A. (2001). "Performance of passive damping systems for the seismic retrofit of steel moment resisting frames subjected to near field ground motions." Earthquake Spectra, 17(3), Pp 427–456.