

Study on Economics of Superstructure of Post-tensioned Highway Bridge in Co-relation with Span to Depth Ratio

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Abstract - Span-to-depth (Slenderness ratio) ratio is an important bridge design parameter that affects structural behavior, construction costs and aesthetics. A study of 86 constant-depth girders indicates that conventional ratios have not changed significantly since 1958. These conventional ratios are now questionable, because recently developed high-strength concrete has enhanced mechanical properties that allow for slenderer sections.

Based on material consumption, cost, and aesthetics comparisons, the thesis determines optimal ratios of a 7-span highway viaduct constructed with high-strength concrete. Two bridge types are investigated: cast-in-situ on false work box-girder and cast in situ solid slabs. Results demonstrate that total construction cost is relatively insensitive to span-to-depth ratio over the following ranges of ratios: 10-35 and 30-45, for the two bridge types respectively. This finding leads to greater freedom for aesthetic expressions because, compared to conventional values (i.e. 18-23 and 22-39), higher ranges of ratios can now be selected without significant cost premiums.

Key Words: Highway Bridge, Slenderness ratio, span to depth ratio, Box girder, Solid slab

1. INTRODUCTION

The bridge design is very complicated design and in the designing of bridge deck the most important parameter is span to depth ratio which is also called as slenderness ratio. This ratio is generally used to fix the superstructure depth and it is chosen during the conceptual designing before any detailed calculation performed. The selection of the ratio at an early stage of the process of design gives the approximate dimension which is necessary for the analysis of bridge girders and cost efficiency and aesthetic merits of the design in comparison with alternative design concept. Span to the depth ratio is generally selected based on the field experience and some values used in previously constructed bridges with satisfactory performance. The ratio can also be determined by optimizing the combinations of ratios and superstructure depth to create cost efficient and aesthetic structure but this requires an iterative process therefore the optimization of the span to

depth ratio in every design concept, it is common to select ratios from the given ranges of conventional values.

The selection of span to depth ratio is generally critical in the design of bridge with girders because the cost of materials and construction of the superstructure is directly affected by span to the depth ratio. For example, using high span to depth ratio reduces the volume of concrete and increases the requirement of prestressing force and simplifies the construction, because in a lighter structure the cost of the bridge is highly dependent on the proportion of the superstructure, so the selection of span to depth ratio is very important for economy. Some proven ranges of span to depth ratios over the past decades given by different organization and Authors for different types of bridges, like; cast in situ box girder, cast in situ slab, and precast segmental box girder shown below in table 1.1,

Table 1.1 Recommended Ratios for Box-Girder

Organization/Author	Year	Span to depth ratio
Leonhardt	1979	12 to 16
ACI-ASCE	1988	25 to 33
Menn	1990	17 to 22
AASHTO	1994	25
Cohn &Lounis	1994	12 to 20
AASHTO-PCI-ASBI	1997	25

Recommended ratios for Cast in Situ Slab

Organization/Author	Year	Span to depth ratio
Leonhardt	1979	18 to 36
ACI-ASCE	1988	24 to 40
Menn	1990	25
AASHTO	1994	37
Cohn &Lounis	1994	22 to 29
Hewson	2003	20

A table 1.1 and 1.2 shows there has been no more changes in the recommended span-to-depth ratio since 1979 although the improvement in material strengths and construction technologies.

2. Literature Review

Miss. P.R. Bhivgade (2001)^[4] in her paper has studied a simply supported Box Girder Bridge for two lane road made up of prestressed concrete which is analysis for moving loads as per Indian Road Congress (IRC:6) specification, Prestressed Code (IS: 1343) and also as per IRC: 18-2000 specifications. The analysis was done by using SAP 2000 14 Bridge Wizard and prestressed with parabolic tendons in which utilize full section. The various slenderness ratios considered to get the proportioning depth at which stresses criteria and deflection criteria get within the permissible limits recommended by IS codes. It is concluded that the basic principles for proportioning of concrete box girder help designer to design the section. For the torsion of superstructure box girder shows better resistance. The various slenderness ratios are carried out for Box Girder Bridges, deflection and stress criteria satisfied well within recommended limits. As the depth of the box girder increases, the prestressing force decreases and the number of cables decrease.

Rajamoori Arun Kumar, B. Vamsi Krishna (2014)^[7] in their paper has studied the practical approach that on a major bridge having 299 meters span, 36 no's of PSC Beams & 8 no's of RCC Beams. The main code that follow in this course is IS: 1343. The title is Code of Practice for Pre-stressed Concrete published by the Bureau of Indian Standards. Remembering that IS: 456 - 2000 which is the Code of Practice for Structural Concrete. Some of the provisions of IS: 456 are also applicable for Pre-stressed Concrete.

The following conclusion were made -

1. Shear force and bending moment for PSC T-beam girder are lesser than RCC T beam Girder Bridge which allow designer to have less heavy section for PSC T-Beam Girder than RCC T-Girder for 24 m span.
2. Moment of resistance of steel for both PSC and RCC has been evaluated and conclusions drawn that PSC T-Beam Girder has more capacity for 24 m and more than 24m of span.

3. PROBLEM IDENTIFICATION

The study of 86 used bridges has been done to presents the variations of span-to-depth ratios. Specifically, the study determines the range of ratios typically used for construction and examines its variations over the past 49 years. Two types of bridge decks are to be analyzed. (i) box-girder bridge deck (ii) solid slab bridge deck

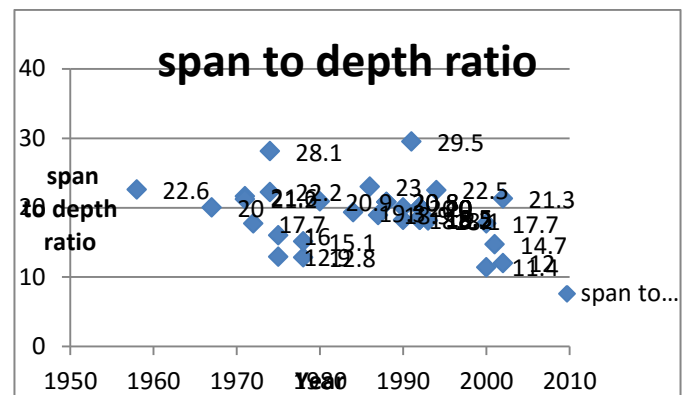
3.1 Some Existing bridges and their span to depth ratio in

3.1.1 Cast in situ Box-Girder Bridge

First 44 cast-in-situ box-girders with constant depth throughout the length are investigated. Table 2-1 provides the basic information of each bridge. Additional information, including the span of the bridges, dimension of girders, detail of designer and references, is given in Appendix A.1. Graph 3-1 shows the variation of span-to-depth ratios with respect to the span lengths and compares these ratios with the recommended values described in chapter 1. Graph 3-2 shows the variation of span to depth ratio with respect to the completion years.

Table no. 2.1 Summary of Cast in Situ Box Girder.

Bridge. No.	Bridge Name	Year	Span to depth ratio
1	Grenz Bridge at Basel	2000	17.7
2	Sart Canal-Bridge	2002	12
3	WeyermannshausBridge	1987	18.9
4	Eastbound Walnut Viaduct	1986	23



Graph 3-1 variation of span to depth ratio of box

Graph 3-1 demonstrates that all 44 cast-in-situ box-girders have span lengths varies between 35.4m and 138 m and demonstrates span-to-depth ratios that ranges varies from 11.4 to 29.5. The graph shows that 42 out of 44 bridges (95%) investigated have span lengths varies from 35m to 75m which is the typical range for constant-depth cast in situ box-girders as suggested by Hewson . You can see in above the frequency plot on top of the graph are bridge numbers that relate each data point to its corresponding bridge in Table 3-1. The red line shows the border of large concentration of bridges that have span-to-depth ratios varies between 17.7 and 22.6. In fact, 34 out of 44 bridges (75%) have ratios within the range of values

suggested by Menn (17 to 22) and Hewson (20) which are based on existing bridges with acceptable performance.

3.1.2 Cast-in-Situ Solid Slab

In addition to cast-in-situ box-girders, 28 cast-in-situ constant-depth solid slab bridges are also investigated.

Table 3.1 Summary of cast in situ solid slab

Bridge No.	Location	Year	Span to Depth Ratio
1	Khandeshwar Bridge	2000	20.3
2	Spadina Ave. Bridge #16, Hwy 401	1967	22.2
3	Spadina Ave. Bridge #18A, Hwy 401	1967	22.2
4	Spadina Ave. Bridge #18B, Hwy 401	1967	22.2

4. Methodology & Analysis

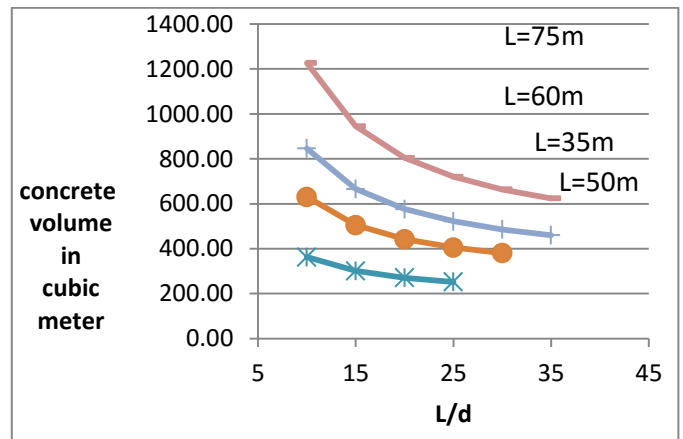
4.1 Analysis Overview

The purpose of this analysis is to compute the amount of prestressing force and the concrete strength needed to satisfy design requirements for bridges with varying span lengths and slenderness ratios. These material consumption results are then used to compute the cost of construction as a function of span-to-depth ratio. By analyzing the variations in construction cost and aesthetic impacts, the study determines the most economical span to depth ratios for different bridge types like box girder bridge, solid slab bridge etc. The two types of post-tensioned bridge are considered ie cast-in-situ box-girder, cast-in-situ solid slab.

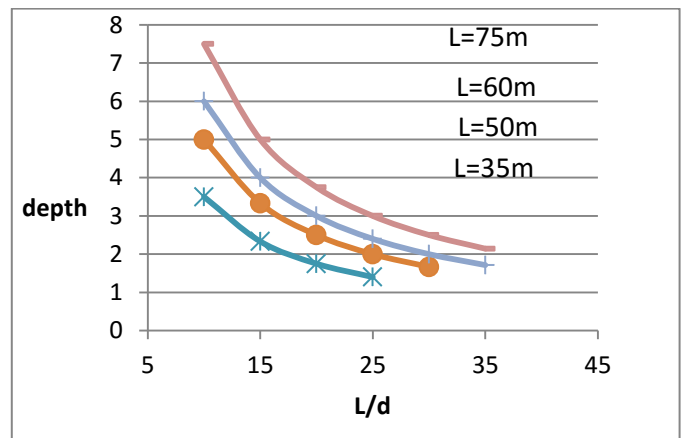
4.2 DESIGN AND ANALYSIS

4.2.1 Analysis of Box Girder Bridge Deck

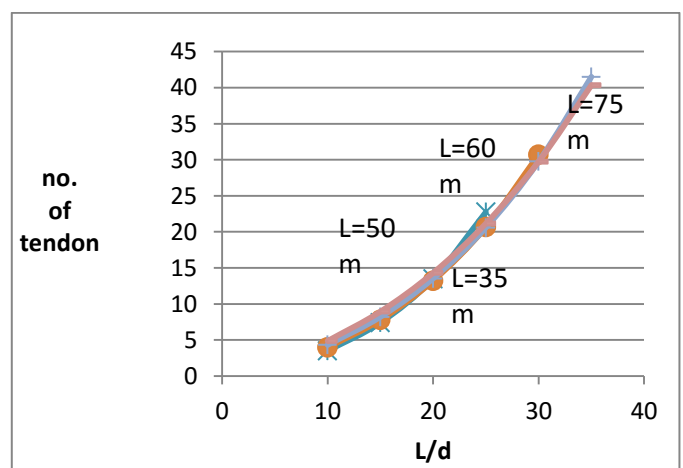
Analysis is performed on 22 cases with span lengths (interior) of 35m, 50m, 60m, and 75 m and slenderness ratios of 10, 15, 20, 25, 30, and 35. This set of span lengths is chosen because cast-in-situ box-girders are economical for spans up to about 100 m according to Menn (1990) and bridges with longer spans need to be haunched in order to reduce self weight. The end spans are made 10 m shorter than interior spans to balance moments along the entire bridge and to make simpler the treatment of prestressing in the study.



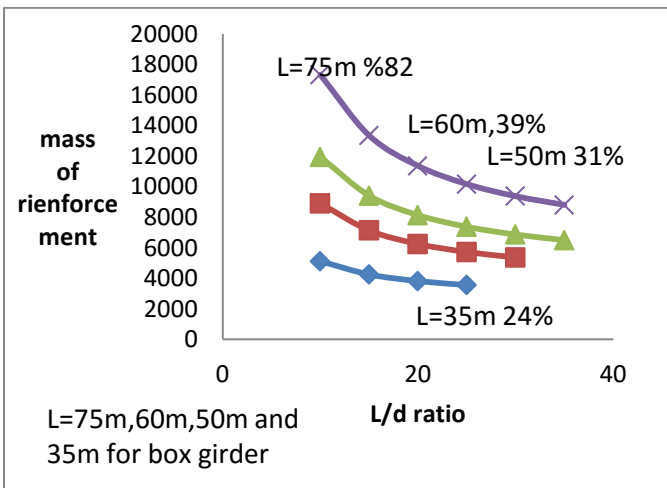
Graph 4.1 Variation of concrete volume with span by depth ratio for different span



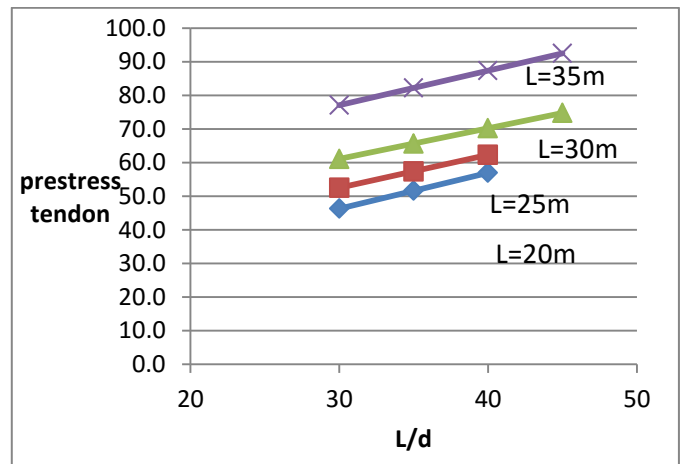
Graph 4.2 Variation of span to depth ratio for different span with depth



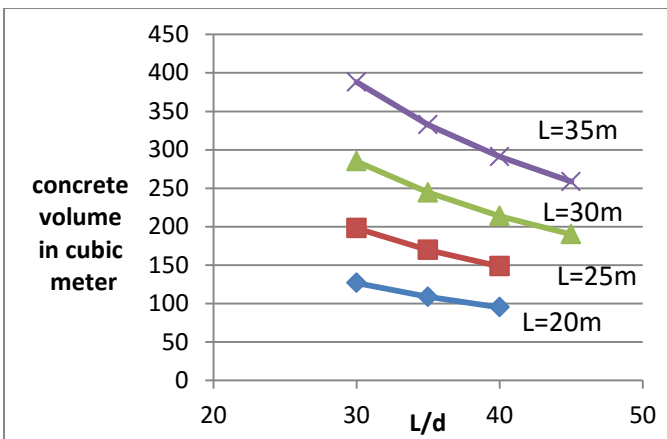
Graph 4.3 Variation of no. of strands with span by depth ratio



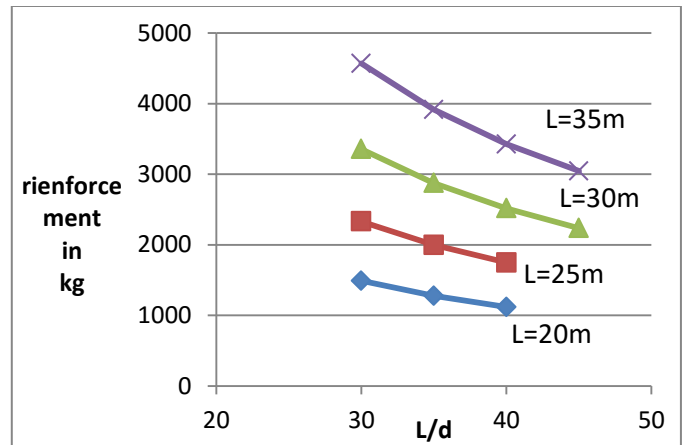
Graph 4.4 Variation of reinforcement with span by depth ratio



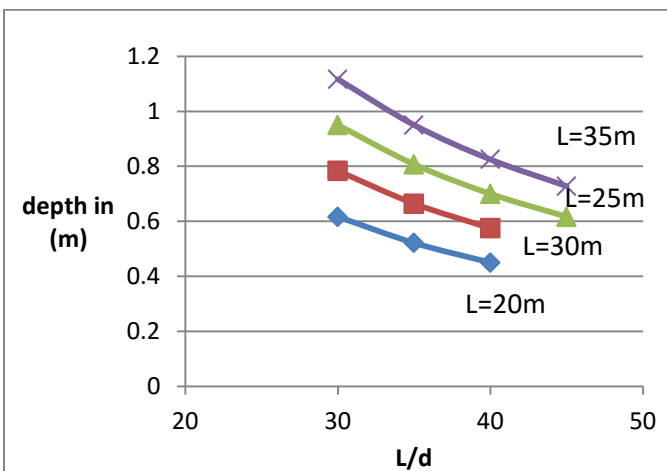
Graph 4.7 Variation of no. of strands with span by depth ratio



Graph 4.5 Variation of concrete volume with span by depth ratio for different span



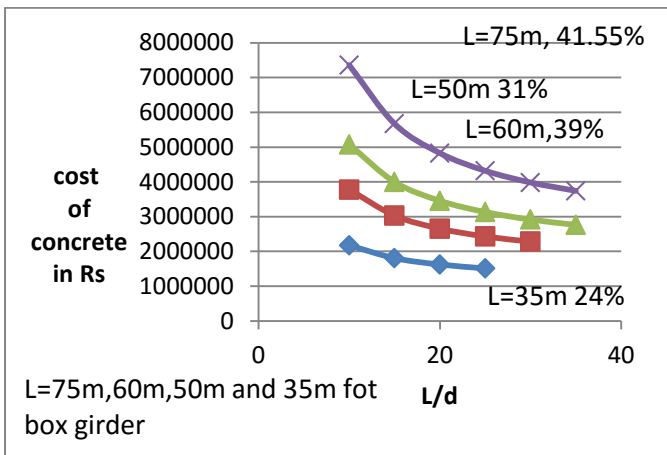
Graph 4.8 Variation of no. reinforcement with span by depth ratio



Graph 4.6 Variation of span to depth ratio for different span with depth

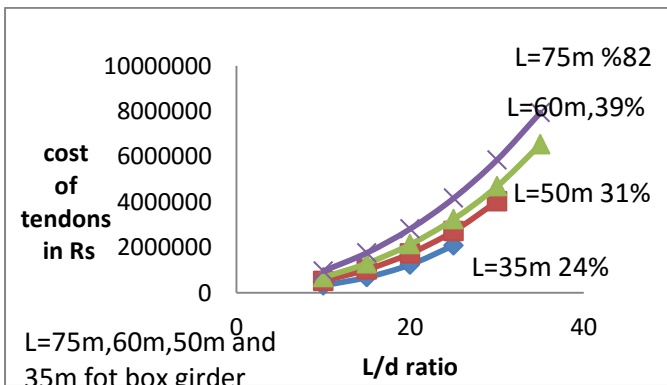
5. COST ANALYSIS

This cost study investigates the changes in superstructure and total cost of construction, which are based on the previously described material consumption results, when slenderness ratio varies. Comparison of these costs reveals the optimal slenderness ratio for each type of bridge. The study also demonstrates the cost benefits of using the optimal ratios instead of conventional ratios.

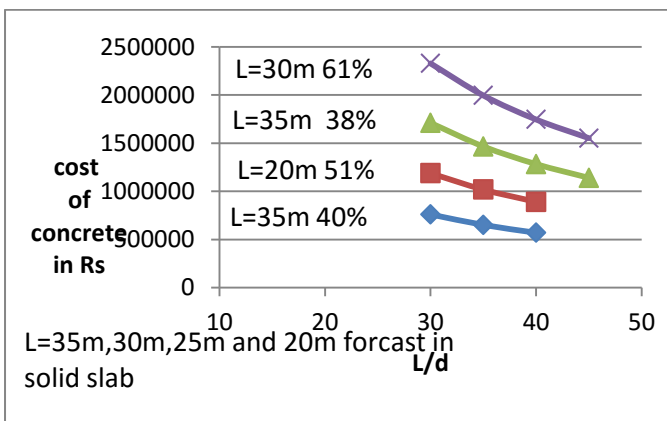


Graph 5.1 variation of cost of concrete with different span with different l/d ratios

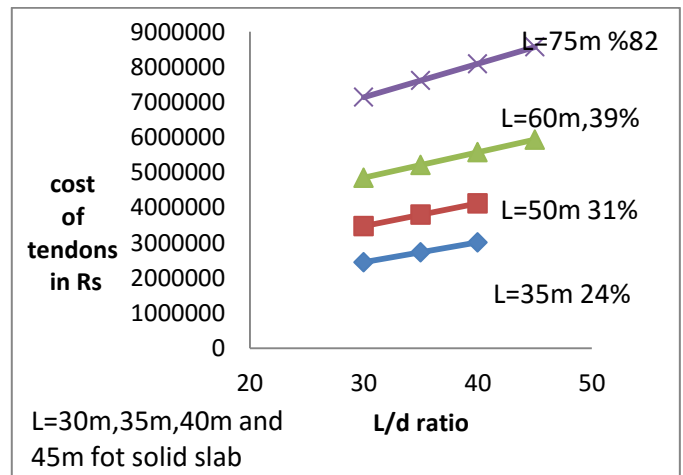
Graph 5.1 summarizes cost of concrete of all the analysis cases calculated. In general concrete cost decreases with increasing span to depth ratio. For box girder, however, cost increases drastically for the very slender cases which require concrete strengths higher than 60Mpa



Graph 5.2 Variation of Cost of Tendons



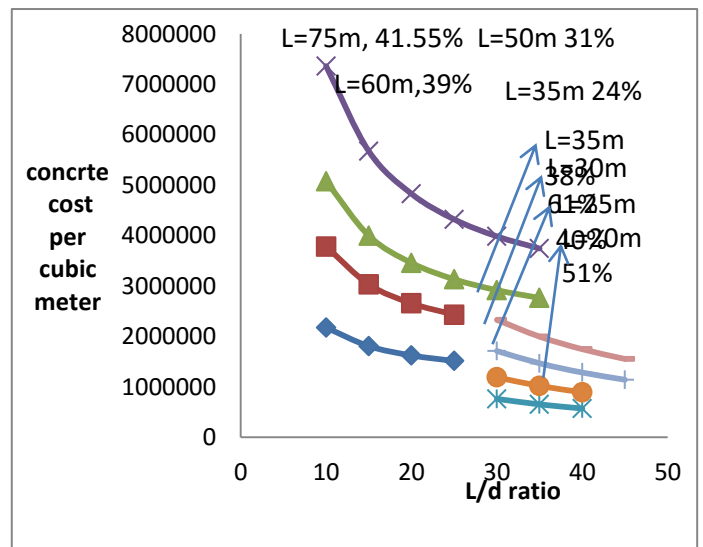
Graph 5.3 variation of cost of concrete with different span with different l/d ratios



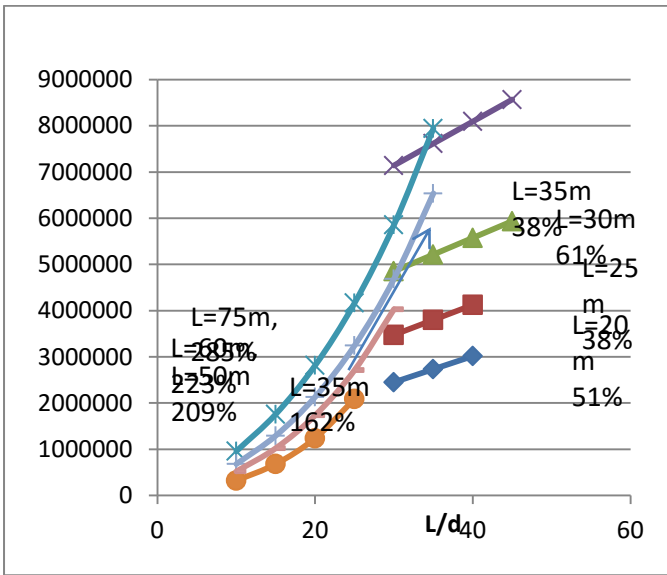
Graph 5.4 variation of cost of tendons

5.1 Comparison of Cost

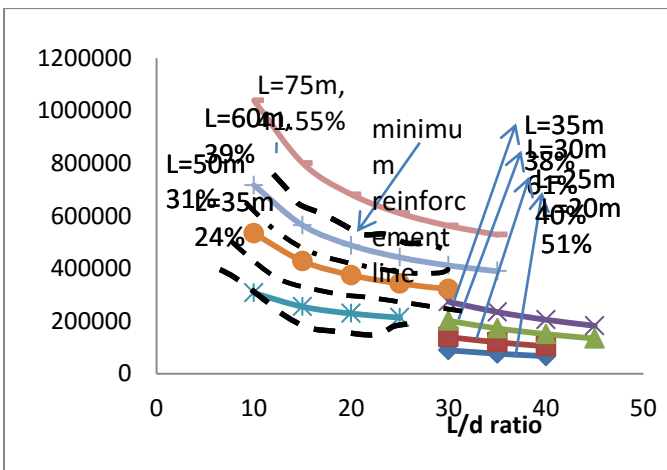
This cost study has been done to know the changes in superstructure and total construction costs, which are based on the previously described material consumption results, when span-to-depth ratio varies. Comparison of these costs reveals the optimal slenderness ratio for each bridge type. The cost study also demonstrates the cost benefits of using the optimal ratios instead of conventional ratios.



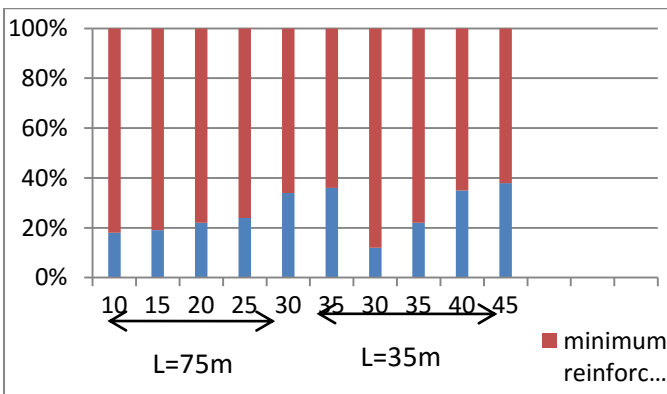
Graph 5.5 Comparison of Cost of Concrete



Graph 5.6 Comparison of Prestressing Tendon Cost

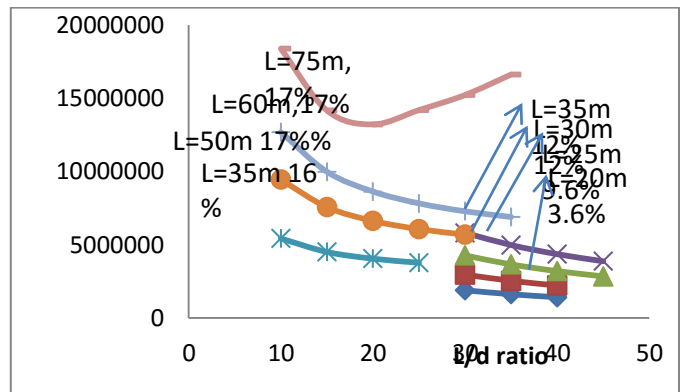


Graph 5.7 Cost comparisons of stirrups and minimum reinforcing steel

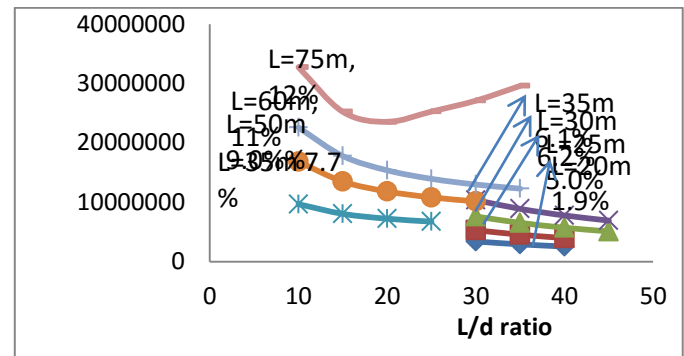


Graph 5.8 Cost distributions of reinforcement and stirrups

5.2 Cost comparison of total Superstructure (including cost of concrete placement)



Graph 5.9 Total concrete of superstructure cost comparison



Graph 5.10 Total construction cost comparison

Table 5.1 Summary of cost study

	Cat in situ box-girder	Cast in situ solid slab
Analysis range of ratios	10 - 35	30 - 50
Typical range of ratios	17.7 - 22.6	22 - 39
Conventional ratio	20.1	30.2
Cost-optimal ratio	25	40
Cost component		
Concrete	-5.1% (41.55%)	-19% (22%)
Prestressing tendon	+28% (285%)	+53% (61%)
Reinforcement steel	-1.3 % (17%)	-5.3 % (5.7%)
Total superstructure	-0.6 % (19%)	-11% (11%)
Total construction cost	-0.4 % (11%)	-5.8 % (6.2%)

6.1 Conclusion

Girder-type bridges have commonly been designed using conventional slenderness ratios which have not changed significantly despite recent development in material strengths and construction technologies. This study determines the optimum slenderness ratios for two types of girder bridges constructed with high-strength concrete: cast-in-situ box-girder and solid slab, the ratios are optimized based on material consumption and total construction cost. The results of this thesis are summarized as follows.

A study of 86 constant-depth girder bridges reveals that the typical ranges of slenderness ratios are 17.7 to 22.6 for cast-in-place box-girder, 22 to 39 for cast-in-situ solid slab, and 15.7 to 18.8 for precast segmental box-girder. The study demonstrates that the ratios for cast-in-situ box-girders have not varied significantly from 1958 to 2007. The study also indicates that cast-in-situ solid slabs constructed after 1975 are mostly voided slabs with slenderness ratios below 25 due to the more stringent code requirements in recent years.

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