

OPTIMIZATION AND ANALYSIS OF CABLE STAYED BRIDGES

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Abstract: This paper deals with the study on optimization of cable stayed bridge with different cable configuration based on connection to the deck and the tower; and the different shapes of pylon (i.e. one axial layer of stays and two laterals of stays) up to failure and evaluates the most suitable configuration of the cable and the tower by using FEM software. The pylons are of two laterals of stays i.e. "A" shape, "Y" shape, "H" shape and one axial layer of stays i.e. circular shape pylon. All the considered shapes of pylon has cross sectional area but is of different shape. The cable configurations are mainly of four types which are based on the connection of cables to the deck and the tower i.e. HARP configuration, FAN configuration, RADIAL configuration, STAR configuration.

Key Words: Cable configurations, shapes of pylon (based on one axial layer of stays and two laterals of stays).

I INTRODUCTION

Cable stayed bridges date back many centuries; the system was used by Egyptians for their sailing ships. Early Chinese people used the cable-stayed system to construct suspension bridges out of hemp rope and iron chains. There are many other examples of ancient cable-stayed bridge systems found around the world.

The first modern bridge structures were a combination of a suspension and stayed system. They were constructed at the end of the eighteen-century in the United States and in England. F. Dischinger identified the need to increase the stress in the cables so as to reduce the sag effect in the stiffness. This advancement gave the impulse to modern cable based structures. In 1955 he built the Stromsund bridge, located in Sweden, which is considered the first modern cable stayed bridge. Another important factor in the evolution of cable-stayed bridges was the employment of superstructure sections that act as a continuous girder along the longitudinal axis. With these improvements, modern cable-stayed bridges became very popular in the last thirty years. Significant illustrated milestones are the Theodor Heuss bridge in 1958, the Schiller-Steg footbridge which was constructed in Germany in 1961, the Maracabio Bridge constructed in Venezuela in 1962.

II EXPERIMENTAL STUDY

1.Harp or parallel system:The cables are parallel to each other and are connected to the tower at different heights. The aesthetics of this kind of configuration are very pleasant. However, the compression in the girder is higher than the others patterns, and the tower is subjected to bending moments (Figure 2.1) .

2.Fan System:This is a modification of the harp system; the cables are connected at the same distance from the top of the tower. The fan system is attractive for a bridge where the longitudinal layout is a single-plane, because the cable slope is steeper, it needs and consequently the axial force in the girder is smaller [7]

3.Radial System:With the radial configuration, all the cables connect to the top of the tower. This is a convenient cable configuration because all the cables have their maximum inclination; therefore the amount of material required in the girder is reduced . However, this configuration may cause congestion problems and the detailing may be complex (figure 2.3).

4.Star System:With the star configuration, all the cables connect to the two opposite end points on the deck. This is a convenient cable configuration because all the cables have their maximum inclination; therefore the amount of material required in the girder is reduced. However, this configuration may cause congestion problems and the detailing may be complex (figure 2.4).

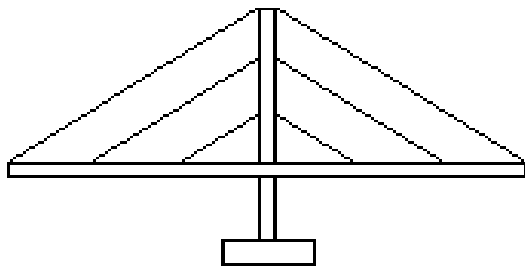


Figure 2.1: Harp System

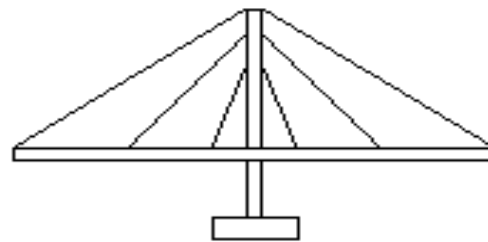


Figure 2.2: Fan System

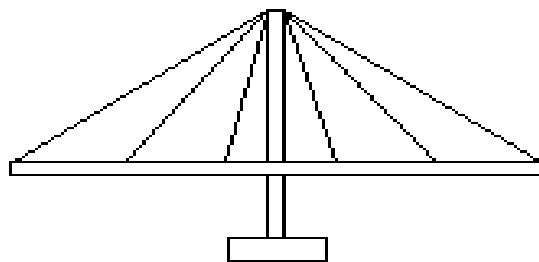


Figure 2.3: Radial System

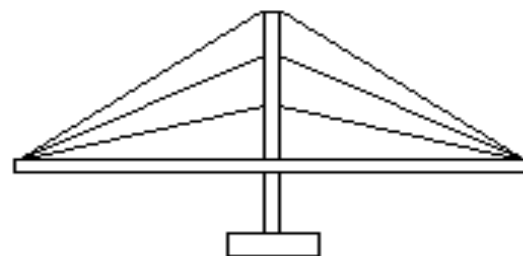


Figure 2.4: Star System

TOWER TYPES:The tower shape is mainly selected for aesthetic reasons, and is refined based on proportions, materials, and restrictions associated with the tower design. A considerable variety of tower shapes exist. In general, the shape of the tower is governed by the required height and the environmental loading conditions, such as seismic zones and wind criteria. The towers are classified according to the basic forms shown in figure 2.5 .

The towers are subjected to axial forces. Thus they must provide resistance to buckling. A more sophisticated analysis of the tower includes the non-linear P-Delta effect. In addition, the tower strength also has to be checked under lateral loads and second-order effects (non-linearity) produced by tension in the cables.Box-sections are most frequently used for the towers. They can be fabricated out of steel or reinforced or prestressed concrete. Concrete towers are more common than steel towers because they allow more freedom of shaping, and are more economical.

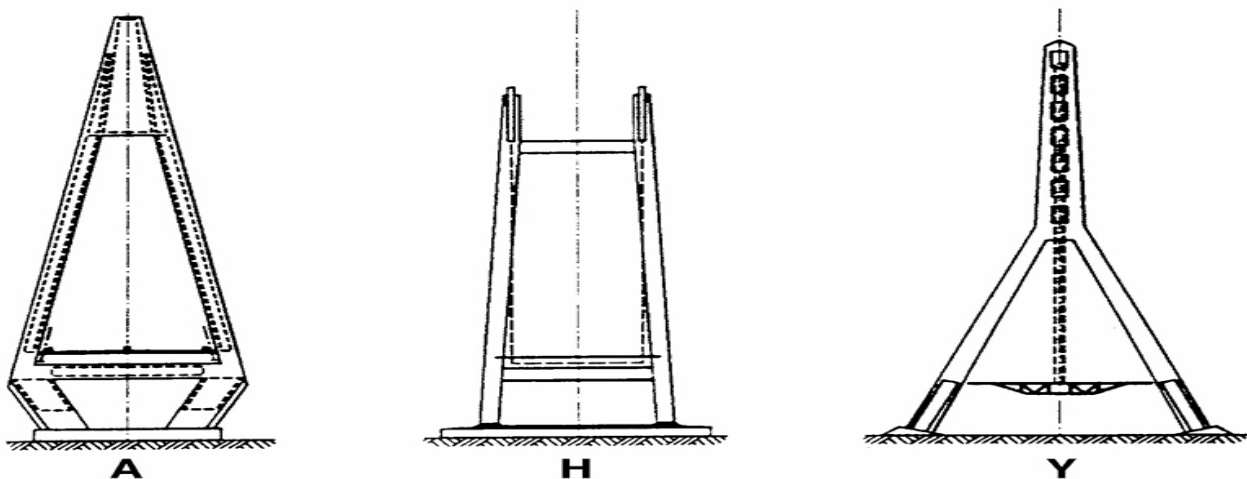


Figure2.5: Tower configuration

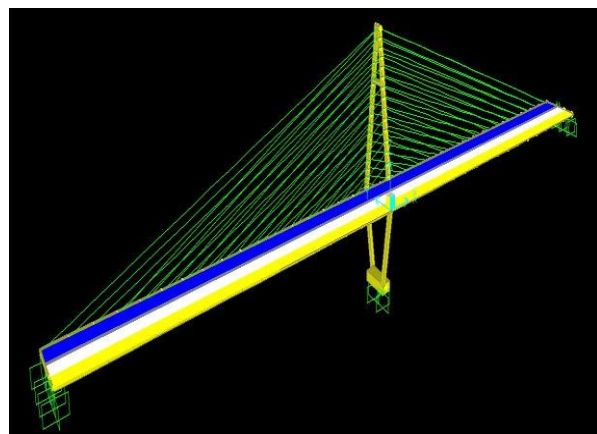
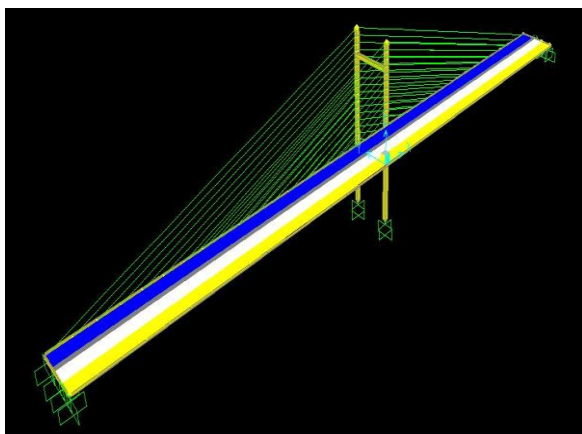
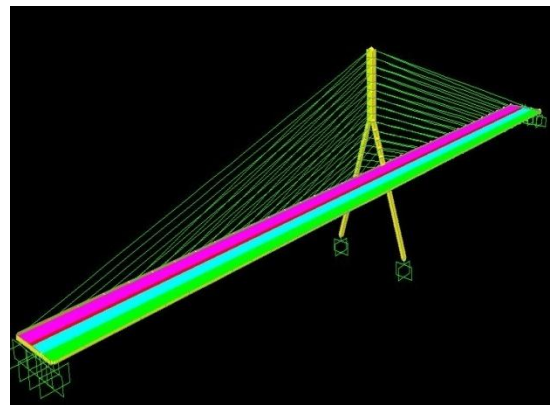
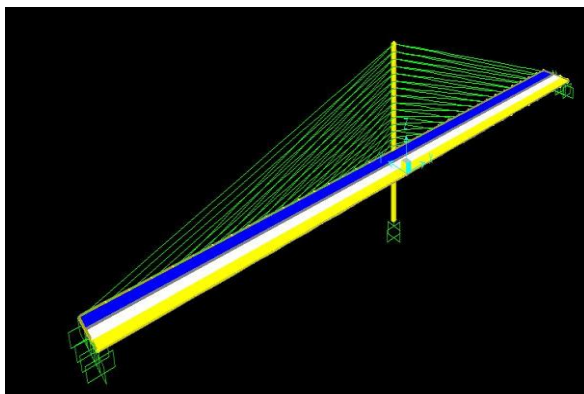
IV EXPERIMENTAL PROCEDURE METHODOLOGY

Analysis of the bridge: The cable stayed bridge can be analysed up to its failure by using software by different pylons is of having same cross sectional area...

- Total span=160m

- Width of the deck=10m
- Width of the road way=7.5m (two lane)
- Number of cables=34
- Spacing of cables=4.7m
- Deck slab thickness=180mm
- Wearing coat thickness=70mm
- Longitudinal girders (2no.s)= 600*800mm
- Cross girders@3mc/c= 450*800mm
- Total height of the pylon = (L/4.7)
- Height of pylon above road way= (L/7)
- Diameter of circular pylon= 1.42m
- Cross section details of rectangular pylon:
- Pylons=1.05*0.75
- Connecting beam at top=0.86*0.75
- Connecting beam at bottom=1.05*0.75

CIRCULAR, A SHAPE, H SHAPE, Y SHAPE PYLON WITH HARP CABLE CONFIGURATION MODELS IN SAP2000



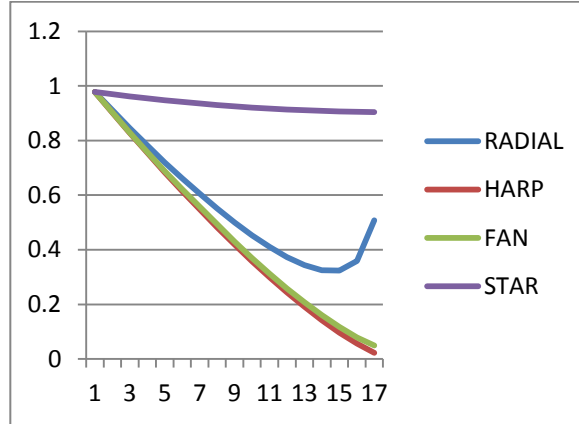
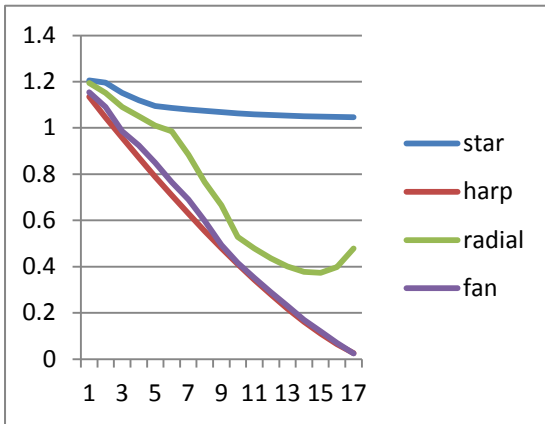
V RESULTS AND DISCUSSION

RESULTS

7.1 GRAPHS B/W VARIOUS ARRANGEMENTS OF CABLE VS SAME PYLON:

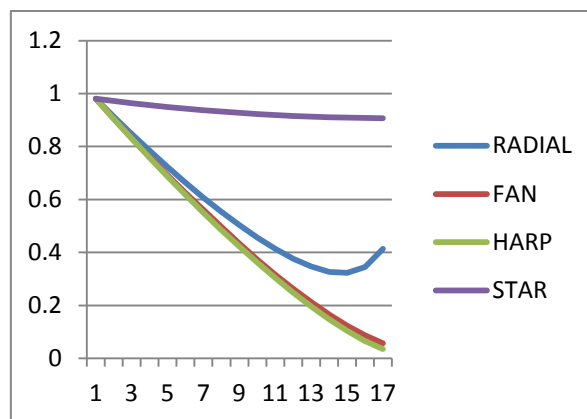
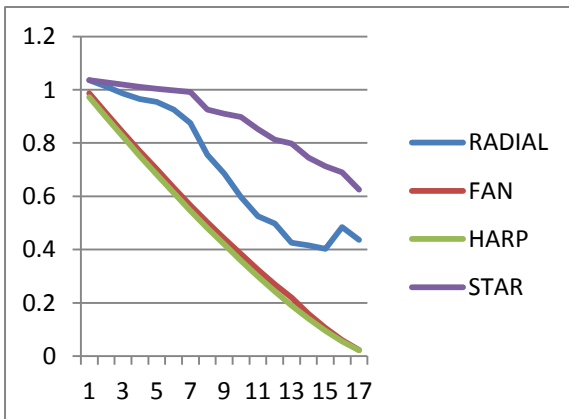
7.1.1 "A" SHAPE PYLON

7.1.2 "H" SHAPE PYLON



7.1.3 "Y" SHAPE PYLON

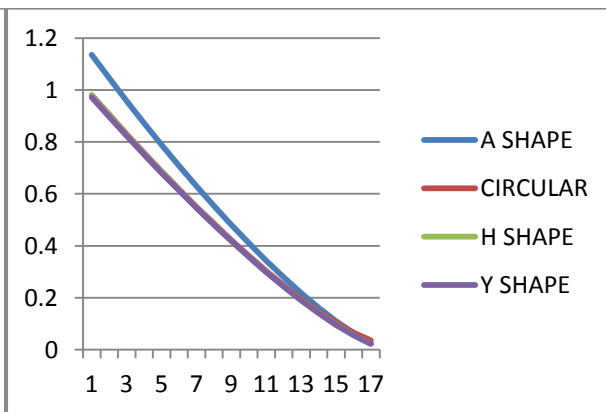
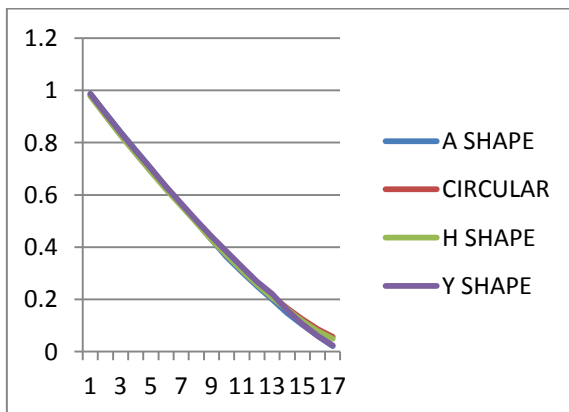
7.1.4 "CIRCULAR" SHAPE PYLON



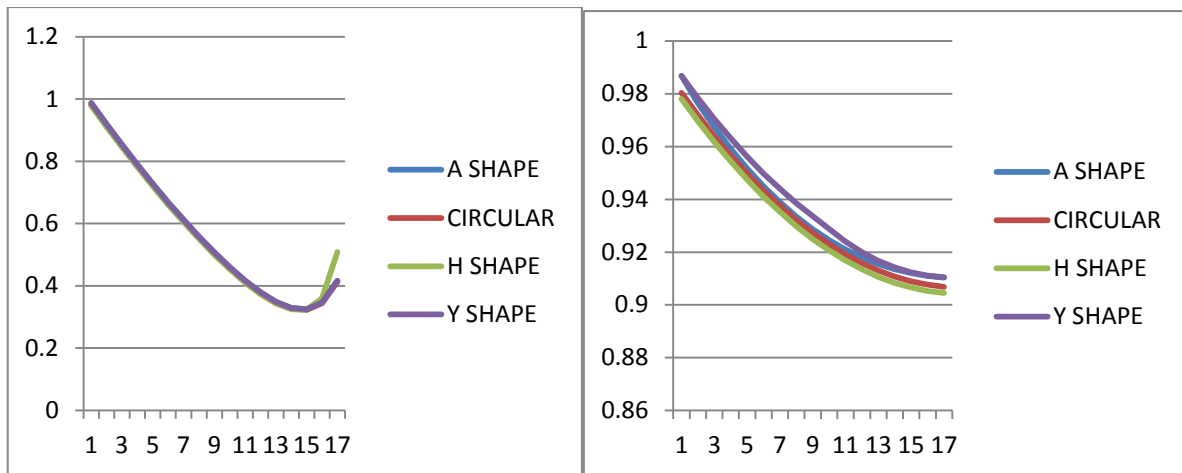
7.2 GRAPHS B/W SAME CABLE CONFIGURATIONS VS DIFFERENT PYLONS

7.2.1 FAN SHAPE CABLE CONFIGURATION

7.2.2 HARP SHAPE CABLE CONFIGURATION



7.2.3 RADIAL SHAPE CABLE CONFIGURATION 7.2.4 STAR SHAPE CABLE CONFIGURATION



ALL GRAPHS ARE PLOTTED B/W CABLE SAG(Y AXIS) VS CABLE NUMBERS(X AXIS)

V) CONCLUSIONS

Based on shape of the tower:

Among all the pylons (i.e. one axial layer of stay and two lateral of stays) the circular and the H shape pylon can has low amount of sag and moment in the cables and the deck because of having the more number of joints comes under non-homogeneity so that the concentration of stress and tension carrying capacity of the cables was not effectively to the other parts of the cable which may lead to maximum concentration in some cables, when compared to circular which has homogeneous member.

Based on cable configuration to the tower and the deck:

The harp and the fan shape configuration is more suitable than star and radial configuration. Because, harp configuration which has evenly spaced to the tower and deck can reduced the sag in the cables. Whereas the fan shape configuration has more less amount of sag when compared to harp shape but the outer cables in the fan configuration may attain more stress concentration, unless providing some dampers to the outer cables (i.e. far away from the tower) is the most suitable configuration. The star and radial configuration may have more sag when compared to other types.

Finally, among all these study the pylons (i.e. one axial layer of stay and two lateral of stays) the circular and the H shape with harp and the fan shape configuration is the best configuration.

REFERENCES

1. STRUCTURAL DESIGN OF CABLE-STAYED BRIDGES -Carlos Miguel Cabeçadas Calado Instituto Superior Técnico - Universidade Técnica de Lisboa, Portugal, Outubro 2011
2. ANALYSIS OF DAMPER EFFECT ON CABLE-STAYED BRIDGE USING FINITE ELEMENT SIMULATION-Chai Mei Hwa* & Siti Aminah Osman on 19 February 2016. Article · December 2015 DOI: 10.17576/jkukm-2015-27-05
3. SEISMIC COMPONENT DEVICES:Agrawal A.K., Amjadian M.Department of Civil and Environmental Engineering, The City College of the City University of New York, 160 Convent Ave.,New York, NY 10031.2011
4. CABLE-STAYED BRIDGES: Man-Chung TangT.Y. Lin International journal on engineering and technology.2007
5. DEAD LOAD ANALYSIS OF CABLE-STAYED BRIDGE - Tao Zhang 1,2 +, ZhiMin Wu 1 2011 International Conference on Intelligent Building and Management
6. "EXPERIMENTAL STUDY ON BRIDGE STAY CABLE VIBRATION MITIGATION USING EXTERNAL VISCOUS DAMPER" (2011). Electronic Theses and Dissertations. Paper 80.

7. PARAMETRIC STUDY OF SHAPES OF PYLON FOR CABLE STAYED BRIDGE -Thomas Blesson B. and S. P. Thakkar
NIRMA UNIVERSITY JOURNAL OF ENGINEERING AND TECHNOLOGY, VOL.2, NO.1, JAN-JUN 2011

8 . A MATHEMATICAL MODEL FOR ASSESSMENT OF MATERIAL REQUIREMENTS FOR CABLE SUPPORTED BRIDGES:
IMPLICATIONS FOR CONCEPTUAL DESIGN. W. J. Lewis, Inz, MSc, PhD,
CEng,FICESchoolofEngineering,UniversityofWarwick-2012 <http://www.sciencedirect.com/science/journal/0141029>

9. DYNAMIC RESPONSE OF PEDESTRIAN BRIDGES/FLOOR VIBRATION AND VARIOUS METHODS OF VIBRATION
REMEDICATION: hung C. Fu, Ph.D., P.E.2009